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## Editorial Note

Technological University Lashio Journal of Research & Innovation (TULSOJRI) has carried out peer-reviewed process in which the identities of the authors and reviewers have not been revealed to each other. As being declared, the main purpose of the Technological University Lashio Journal of Research & Innovation (TULSOJRI) is to promote the creative thinking skill and innovation of our teachers in their respective fields. The objectives are to integrate the analytical skill with theoretical and practical knowledge and to foster innovations in science and engineering fields for our development.

This journal is open to all scientists, researchers and engineers to submit their original research papers which were not previously published. On behalf of TULSOJRI team, I would like to sincerely express our appreciation to all the authors in this journal. We gratefully acknowledge the contribution of all the editors, production editors and the collaboration of the reviewers who devoted their invaluable time to reviewing process.

This journal has been officially permitted to publish according to the guidelines of the Department of Higher Education. All the papers in this volume were double-reviewed and approved of free from plagiarism.

Only 33% out of over 450 submitted papers are accepted to be published in this journal. I would like to strongly encourage the authors whose papers have not been included in this volume to keep on trying.

Dr. Tin San

Executive Editor

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# Technological University Lashio Journal of Research & Innovation (TULSOJRI)

Volume 01, Issue 04, September 2020

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## Application of Triangular Elements Method for Poisson's Equation

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**ABSTRACT:** In this paper, we introduced the finite element idealization and triangular element idealization. And then, especially triangular element method for Poisson's equations are thoroughly discussed with boundary conditions and compared with the finite element solutions, finite difference solutions and exact solutions of Poisson's equations. It is the purpose to present brief derivations of the finite element approximation describing a discrete model and wrote the computer program of triangular elements solution by using the C++.

**KEYWORDS:** Finite Element Method, Triangular Element Method, Finite Difference Method, Variational Formulation, C++.

### 1. INTRODUCTION

Finite element method is summation of a piecewise elements in total element E approximation to the exact solution. Let us consider the differential problem  $L\phi = f$  in R, with  $B\phi = g$  on C, the bounding C subset R. In this problem, each element  $\phi^e(x, y)$  approximations to the exact solution  $\phi(x, y)$  is  $\phi(x, y) = \sum_e \phi^e(x, y)$  where the summation is total elements E,  $(e = 1, 2, \dots, E)$  and outside of the element is zero.  $\phi^e(x, y) = N^e(x, y)\delta^e$  where  $N^e(x, y)$  is called the shape function and  $\delta^e \{\phi_1, \phi_2, \phi_3, \dots\}$  is called the nodal displacement. Variational formulation [1] of Poisson's equation with boundary condition is

$$I[\phi] = \sum_e \iint_{R^e} \left\{ k \left( \frac{\partial \phi}{\partial x} \right)^2 + k \left( \frac{\partial \phi}{\partial y} \right)^2 - 2\phi f \right\} dx dy + \sum_e \int_{C^e} \{ \sigma(\phi^e)^2 - 2\phi^e h \} ds.$$

### 2. TRIANGULAR ELEMENTS DESCRETIZATION

Rectangular elements are not appropriate on the irregular boundaries because it is not easy to by the rectangular shape to the boundary geometry with elements as show in Figure 1. But triangular element is closely to the boundary than rectangular elements by a polygonal arc as show in Figure 2. Therefore, triangular element approximation is better than rectangular element approximation. If the mesh points are a larger number, the accuracy of solution is more and more better. [1]

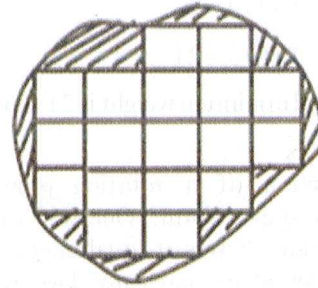


Fig1. A typical geometry suitable for approximation with rectangular elements

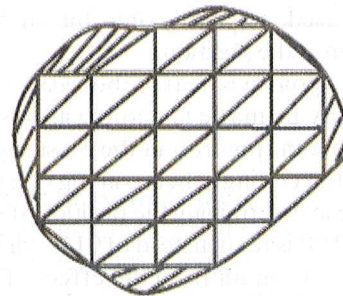


Fig 2. Approximation with triangular elements using the same number of nodes.

Consider the triangular element of the position P with the global coordinates  $(x, y)$  and local coordinates or triangle coordinates or area coordinates  $(L_1, L_2, L_3)$  as shown in Figure 3.

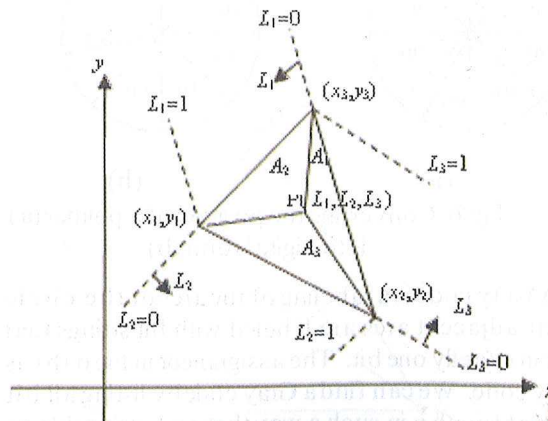


Fig 3. Area coordinates for a triangular element  $L_1 = \frac{A_1}{A}, L_2 = \frac{A_2}{A}, L_3 = \frac{A_3}{A},$  (1)



where  $A_1, A_2, A_3$  are the areas in the triangle  $A$  as shown in Figure 3 but three coordinates are not independent and they satisfied the equation  $L_1 + L_2 + L_3 = 1$ . (2)

The relation between the global coordinates  $(x, y)$  and the triangular coordinates  $(L_1, L_2, L_3)$  is

$$x = L_1 x_1 + L_2 x_2 + L_3 x_3 \quad (3)$$

$$y = L_1 y_1 + L_2 y_2 + L_3 y_3 \quad (4)$$

and it is obtained  $L_1$  in terms of  $x$  and  $y$  is

$$L_1 = \frac{a_1 + b_1 x + c_1 y}{2A} \quad (5)$$

where the area  $A$  is given by

$$A = \frac{1}{2} \begin{vmatrix} 1 & x_1 & y_1 \\ 1 & x_2 & y_2 \\ 1 & x_3 & y_3 \end{vmatrix} \quad (6)$$

and  $x_i, y_i$  are nodal coordinates and

$$\begin{aligned} a_1 &= x_2 y_3 - x_3 y_2, & a_2 &= x_3 y_1 - x_1 y_3 \\ b_1 &= y_2 - y_3, & b_2 &= y_3 - y_1 \\ c_1 &= x_3 - x_2, & c_2 &= x_3 - x_1, \dots \end{aligned} \quad (7)$$

The derivatives of  $L_i$  are

$$\frac{\partial L_i}{\partial x} = \frac{b_i}{2A} \quad \text{and} \quad \frac{\partial L_i}{\partial y} = \frac{c_i}{2A} \quad (8)$$

where  $a_i, b_i$  and  $c_i$  are constants. The integral involving the area coordinates [1] is.

$$\iint_A L_1^m L_2^n L_3^p dx dy = \frac{2A m! n! p!}{(m+n+p+2)!} \quad (9)$$

An element has three nodes with one degree of freedom at each node, the displacement variation throughout the element is linear form.

$$\phi^e(x, y) = a_0 + a_1 x + a_2 y \quad (10)$$

By using the nodal displacement and interpolating through the element gives

$$\begin{aligned} \phi^e(x, y) &= N^e(x, y) \delta^e \\ \phi^e(x, y) &= [L_1 \ L_2 \ L_3] \{\phi_1 \ \phi_2 \ \phi_3\} \end{aligned} \quad (11)$$

The element stiffness matrix is given by

$$k_{ij}^e = \iint_A k \left( \frac{\partial L_i}{\partial x} \frac{\partial L_j}{\partial x} + \frac{\partial L_i}{\partial y} \frac{\partial L_j}{\partial y} \right) dx dy$$

$$k_{ij}^e = \iint_A k \left( \frac{b_i b_j}{4A^2} + \frac{c_i c_j}{4A^2} \right) dx dy \quad (12)$$

In the special case  $k = 1$ ,

$$k_{ij}^e = \frac{1}{4A} (b_i b_j + c_i c_j)$$

The element force vector is given by

$$f_i^e = \iint_A L_i f(x, y) dx dy \quad (13)$$

terms of  $L_1, L_2, L_3$  and hence  $f_i^e$  may be obtained. If the element boundary side 1-2 as show in Figure 4.

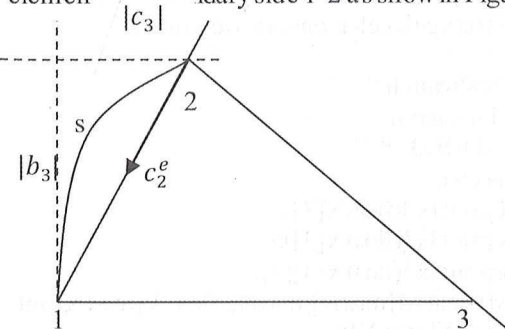


Fig 4. Boundary element with side 3-1 approximating the boundary

$$\begin{aligned} L_3 &= 0 \quad \text{and} \quad s = (b_3^2 + c_3^2)^{1/2} L_1 \\ \text{so that} \quad ds &= (b_3^2 + c_3^2)^{1/2} dL_1 \\ \text{since} \quad L_2 &= 1 - L_1 \end{aligned}$$

$$\bar{k}^e = \int_{c_2} \sigma(s) N^e N^e ds \quad (14)$$

$$\bar{f}^e = \int_{c_2} h(s) N^e ds \quad (15)$$

$$\bar{k}^e = \int_0^1 \sigma \begin{bmatrix} L_1^2 & L_1 - L_1^2 & 0 \\ L_1 - L_1^2 & (1 - L_1)^2 & 0 \\ 0 & 0 & 0 \end{bmatrix} (b_3^2 + c_3^2)^{1/2} dL_1 \quad (16)$$

$$\bar{f}^e = \int_0^1 h \{L_1 \ 1 - L_1 \ 0\} (b_3^2 + c_3^2)^{1/2} dL_1 \quad (17)$$

These results will now be used to obtain a two-element solution using a single rectangular element.

### 3. TRIANGULAR ELEMENTS SOLUTION FOR POISSON'S EQUATION

Consider the problem  $-\nabla^2 \phi = 2(x + y) - 4$  in the square whose vertices are at  $(0,0), (1,0), (1,1), (0,1)$ . The boundary condition are  $\phi(0,y) = y^2, \phi(1,y) = 1-y, \phi(x,0) = x^2, \phi(x,1) = 1-x$ .

The problem has symmetry about the line  $y = x, \frac{\partial \phi}{\partial n} = 0$

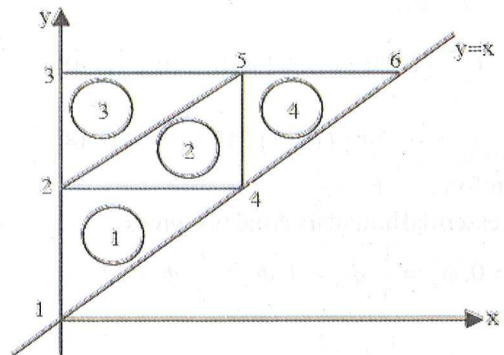


Fig 5. Four triangular element

The element stiffness matrices may be obtained directly from equation (12) and area of each triangle 1/8. Then

$$k^1 = \begin{bmatrix} 1 & 4 & 2 \\ 1/2 & 0 & -1/2 \\ 0 & 1/2 & -1/2 \\ -1/2 & -1/2 & 1 \end{bmatrix} \begin{matrix} 1 \\ 4 \\ 2 \end{matrix}$$

$$k^2 = \begin{bmatrix} 5 & 2 & 4 \\ 1/2 & 0 & -1/2 \\ 0 & 1/2 & -1/2 \\ -1/2 & -1/2 & 1 \end{bmatrix} \begin{matrix} 5 \\ 2 \\ 4 \end{matrix}$$

$$k^3 = \begin{bmatrix} 2 & 5 & 1 \\ 1/2 & 0 & -1/2 \\ 0 & 1/2 & -1/2 \\ -1/2 & -1/2 & 1 \end{bmatrix} \begin{matrix} 2 \\ 5 \\ 1 \end{matrix}$$



$$k^4 = \begin{bmatrix} 4 & 6 & 5 \\ 1/2 & 0 & -1/2 \\ 0 & 1/2 & -1/2 \\ -1/2 & -1/2 & 1 \end{bmatrix} \begin{matrix} 4 \\ 6 \\ 5 \end{matrix}$$

The element force vector is given by equation (13) as

$$f_i = \iint_A L_i [2(x+y) - 4] dx dy, \quad i = 1, 2, 3$$

where 1, 2, 3 is the local nodal numbering defined in Fig 3. Then using equations (3) and (4)

$$f_i = \int_A L_i [2L_1(x_1 + y_1) + 2L_2(x_2 + y_2) + 2L_3(x_3 + y_3) - 4] dx dy$$

$$f_i = \frac{(x_i + y_i)}{24} + \frac{(x_j + y_j)}{48} + \frac{(x_k + y_k)}{48} - \frac{1}{6}$$

The only node without an essential boundary condition is node 4, thus only the equation associated with node 4 is assembled. This equation is

$$\sum_{j=1}^6 K_{1j} \phi_j = F_4$$

where K and F are the over-all stiffness and force matrices respectively. Row 4 of K is [0 -1 0 2 -1 0]; and  $F_4 = f_2^1 + f_3^2 + f_1^4$ , where the subscribe refer to the local nodal numbers of Figure 3.

$$f_2^1 = (\frac{1}{2} + \frac{1}{2}) / 48 + (0 + \frac{1}{2}) / 48 + (0 + 0) / 48 - \frac{1}{6} = -\frac{11}{96}$$

$$f_3^2 = (\frac{1}{2} + \frac{1}{2}) / 24 + (\frac{1}{2} + 1) / 48 + (0 + \frac{1}{2}) / 48 - \frac{1}{6} = -\frac{8}{96}$$

$$f_1^4 = (\frac{1}{2} + \frac{1}{2}) / 24 + (1 + 1) / 48 + (\frac{1}{2} + 1) / 48 - \frac{1}{6} = -\frac{5}{96}$$

Therefore,  $F = -\frac{1}{4}$

The essential boundary condition gives

$$\phi_1 = 0, \phi_2 = \frac{1}{4}, \phi_3 = 1, \phi_5 = \frac{1}{2}, \phi_6 = 0$$

so that

$$-\frac{1}{4} + 2\phi_4 - \frac{1}{2} = -\frac{1}{4}$$

which gives

$$\phi_4 = \frac{1}{4}$$

The solution at  $(\frac{1}{4}, \frac{1}{4})$ ,  $(\frac{1}{4}, \frac{3}{4})$  and  $(\frac{3}{4}, \frac{3}{4})$  is found by linear interpolation between nodes 1 and 4, 2 and 5, 4 and 6 respectively.

#### 4. FINITE DIFFERENT SOLUTION FOR POISSON'S EQUATION

By using the finite different method for Poisson's equation

$$U(x+h,y) + U(x-h,y) + U(x,y+h) + U(x,y-h) - 4U(x,y) = h^2 f(x,y)$$

$$\text{For } P_{11}, u_{21} + u_{01} + u_{12} + u_{10} - 4u_{11} = 0.0625 \{2(1/4 + 1/4) - 4\}$$

$$\text{For } P_{12}, u_{22} + u_{02} + u_{13} + u_{10} - 4u_{11} = 0.0625 \{2(1/4 + 1/2) - 4\}$$

$$\text{For } P_{13}, u_{23} + u_{03} + u_{14} + u_{12} - 4u_{13} = 0.0625 \{2(1/4 + 3/4) - 4\}$$

$$\text{For } P_{21}, u_{31} + u_{11} + u_{22} + u_{20} - 4u_{21} = 0.0625 \{2(1/2 + 1/4) - 4\}$$

$$\text{For } P_{22}, u_{32} + u_{12} + u_{23} + u_{21} - 4u_{22} = 0.0625 \{2(1/2 + 1/4) - 4\}$$

$$\text{For } P_{23}, u_{33} + u_{13} + u_{24} + u_{22} - 4u_{23} = 0.0625 \{2(1/2 + 3/4) - 4\}$$

$$\text{For } P_{31}, u_{41} + u_{21} + u_{32} + u_{30} - 4u_{31} = 0.0625 \{2(3/4 + 1/4) - 4\}$$

$$\text{For } P_{32}, u_{42} + u_{22} + u_{33} + u_{31} - 4u_{32} = 0.0625 \{2(3/4 + 1/2) - 4\}$$

$$\text{For } P_{33}, u_{43} + u_{23} + u_{34} + u_{32} - 4u_{33} = 0.0625 \{2(3/4 + 3/4) - 4\}$$

By using the Gauss-Seidel Method,

$$u_{11} = 0.203125, \quad u_{12} = 0.2890625, \quad u_{13} = 0.484375$$

$$u_{22} = 0.28125, \quad u_{23} = 0.335925, \quad u_{33} = 0.265625$$

#### 5. COMPARISON OF THE SOLUTIONS FOR POISSON'S EQUATION

In the above mention results compared with the corresponding results using triangular elements solutions, finite difference solutions and exact solutions at the same point. If we want to find the another solution of another nodes, element solution is easy to know the solution of the another node but finite difference solution is second time plot the mesh point and calculate the next time.

Table 1. Comparison of the Solutions for Poisson's Equation

(x, y)	(1/4, 1/4)	(2/4, 2/4)	(1/4, 3/4)	(3/4, 3/4)
Four triangular elements solutions	0.125	0.25	0.375	0.125
Finite difference solutions	0.203125	0.2890625	0.484375	0.265625
Exact Solutions	0.094	0.25	0.25	0.281

#### 6. C++ PROGRAM OF TRIANGULAR ELEMENTS SOLUTION FOR POISSON'S EQUATION

The C++ program for solving Poisson's equation, using four triangular elements are described.

```
#include<iostream.h>
#include<iomanip.h>
#define ENODE 3
#define maxN 6
ostream &print1x3(float x[3]);
ostream &print3x1(float x[3]);
ostream &print3x3(float x[3][3]);
void MVM(float M[maxN][maxN], float V[maxN], int K, int L, float Y[maxN]);
void MIV(float M[maxN][maxN], int n);
void OutText(float M[6][6], float V[6], int r);
```



```
void TriK(float K[3][3], float f[3], float x[3], float y[3]){
```

```
    float a[3],b[3],c[3],A;
    int i,j,k;
    a[0]=x[1] * y[2] - x[2] * y[1];
    b[0]=y[1] * y[2];
    c[0]=x[2] - x[1];

    a[1]=x[2] * y[0] - x[0] * y[2];
    b[1]=y[2] * y[0];
    c[1]=x[0] - x[2];

    a[1]=x[2] * y[0] - x[0] * y[2];
    b[1]=y[2] * y[0];
    c[1]=x[0] - x[2];

    a[2]=x[0] * y[1] - x[1] * y[0];
    b[2]=y[0] * y[1];
    c[2]=x[1] - x[0];

    A=0;
    for(i=0; i<3; i++){
        A += a[i]/2;
        j = (i+1) % 3;
        k=(i+2)%3;
        f[i] = (x[i]+y[i])/3.0 + (x[j]+y[j])/6.0
+ (x[k]+y[k])/6.0 - 4/3.0;
    }
    for(i=0; i<3; i++){
        f[i] *= A;
        for(j=0; j<3; j++)

            K[i][j]=1/4.0/A*(b[i]*b[j]+c[i]*c[j]);
    }
```

```
int main(void){
    float x[maxN]={0,0,0.5,0.5,1.0}, xx[3],
    y[maxN]={0,0.5,1,0.5,1,1}, yy[3],f[3],

    u[maxN][2]={{0,0},{0,0.25},{0,1.0},{1,0},{0,0.5},{0,0}},
    K[ENODE][ENODE],

    Ks[maxN][maxN],Fs[maxN],Un[maxN];

    int
    elem[4][ENODE]={{0,3,1},{4,1,3},{1,4,2},{3,5,4}};
    int n=maxN,en=4;
    int i,j,k;
    //Clear Assemblage Matrices
    for(i=0; i<n; i++)
    for(j=0; j<n; j++)
        Ks[i][j]=Fs[j]=0;

    for(i=0; i<en; i++){
        for(j=0; j<ENODE; j++){
            xx[j]=x[elem[i][j]];
            yy[j]=y[elem[i][j]];
        }
        //Element Stiffness Matrix
        TriK(K,f,xx,yy);
        print 1x3(f) << endl;
        print3x3(K) << endl;
```

Force Matrix

```
        for(j=0; j<ENODE; j++){
            for(k=0; k<ENODE; k++)
                Ks[elem[i][j]][elem[i][k]] += K[j][k];
            Fs[elem[i][j]] += f[j];
        }
    }
    cout<<"Assembled equations (Before Boundary Condition)" << endl;
    OutText(Ks,Fs,n);
    //Modify Stiffness Matrix and Force Vector
    for(i=0; i<n; i++){
        if(u[i][0]==0){
            for(j=0; j<n; j++)
                Ks[i][j]=0;
            Ks[i][i]=1;
            Fs[i]=u[i][1];
        }
    }
    cout<<"Assembled equations " << endl;
    OutText(Ks,Fs,n);
    //Inverse Matrix MIV(Ks,n);
    //Ca lulate Displacement Vector
    MVM(Ks,Fs,n,n,Un);
    cout<<"(.25,.25 .5,.5 .75,.75)" << endl;
    cout<<setw(9) <<
    (Un[0]+Un[3])/2.0 << setw(9) <<
    (Un[3]+Un[3])/2.0 << setw(9) <<
    (Un[1]+Un[4])/2.0 << setw(9) <<
    (Un[3]+Un[5])/2.0 << endl;
    return 0;
}
ostream &print1x3(float x[3]){
    int i;
    cout<<setw(3);
    for(i=0; i<3; i++)
        cout<<setw(8) << x[i] << " ";
    return cout;
}
ostream &print3x1(float x[3]){
    int i;
    cout<<setw(3);
    for(i=0; i<3; i++)
        cout<<setw(8) << x[i] << endl;
    return cout;
}
ostream &print3x3(float x[3][3]){
```



```

int i;
for(i=0;i<3;i++)
    print 1x3(x[i])<< endl;
return cout;
}
void OutText(float M[6][6], float V[6], int r){
int i,j;
cout<<setiosflags(ios::fixed);
cout<<setprecision(4);
cout<<endl;
for(i=0;i<r;i++){
for(j=0;j<r;j++){
cout<<setw(8)<<M[i][j]<<"
";
cout<<" | " <<setw(8)<<V[i] <<"
<=< f" << i << endl;
}
}
void MVM(float M[maxN][maxN], float V[maxN],
int K, int L, float Y[maxN]){
float X;
int i,j;
for(i=0; i<K ; i++){
X=0.0;
for(j=0; j<L ; j++)
X += M[i][j] * V[j];
Y[i]=X;
}
}
void MIV(float M[maxN][maxN],int n){
#define FORJN for(j=0;j<n;j++)
#define FORKN for(k=0;k<n;k++)
#define FORIN for(i=0;i<n;i++)
#define M(i,j) M[(i)][(j)]

int i,j,k;
float KII;
FORKN{
KII=M(k,k);
M(k,k)=1;
FORJN
M(k,j)/=KII;
FORIN{
if(i!=k){
KII=M(i,k);
M(i,k)=0;
FORJN
M(i,j) -=
M(k,j) * KII;
}}}}

```

Output of the element stiffness matrices

```

**
Solution
-0.135 -0.115 -0.125
0.5 0 -0.5
0 0.5 -0.5
-0.5 -0.5 1

```

```

-0.073 -0.094 -0.083
0.5 0 -0.5
0 0.5 -0.5
-0.5 -0.5 1

```

```

-0.094 -0.073 -0.083
0.5 0 -0.5
0 0.5 -0.5
-0.5 -0.5 1

```

```

-0.052 -0.031 -0.042
0.5 0 -0.5
0 0.5 -0.5
-0.5 -0.5 1

```

Output of the assembled equations (Before boundary conditions)

```

0.5 -0.5 0 0 0 0 -0.135
-0.5 2 -0.3 -1 0 0 -0.115
0 -0.5 1 0 -0.5 0 -0.125
0 -1 0 2 -1 0 -0.135
0 0 -0.5 -1 2 -0.5 -0.115
0 0 0 0 -0.5 0.5 -0.115

```

```

1 0 0 0 0 0 0
0 1 0 0 0 0 0
0 0 1 0 0 0 0
0 0 0 1 0 0 0
0 0 0 0 1 0 0
0 0 0 0 0 1 0
0.135 0.115 0.125 0.135 0.115 0.115
-0.25 -0.5 0.375 -0.25

```

Above the mention result of The C++ program for solving Poisson's equation and finite element solution are the same. We using the C++ program, we are reduced to time limiting and easy to find the another solution.

7. CONCLUSIONS

In the above mentioned result, triangular elements solution is better than the finite different solution for Poisson's equation. And then, triangular elements solution and C++ program solution are the same for this problems. Finite element approximation for field problems improvements made by refining the finite element mesh. Higher-order elements are get a better polynomial approximation but integral involved would be necessary to use numerical integration. Using this program, we can easy to know the other points of this problem.

If we want to another solution or a another point, finite element method solution is easy to replace at these points but finite different method is next time will mesh and calculate.



It is reasonable to expect that the proposed finite elements model is a very good approximation of the continuum. This method is now very widely used and forms the basis of most calculations

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